SITE INVESTIGATION
FACTUAL REPORT

Report No:	SI-783212
Client:	Sedgwick International UK - Maidstone
Site:	1 Wheatstone Road Cambridgeshire
Client Ref:	10051341
Date of Visit:	3/1/2024

East Building, Cable Drive, Walsall, WS2 7BN

☎ 0333 016 4477
✉ siinstructions@cet-uk.com
💻 www.cet-uk.com

CET is the trading name of CET Structures Ltd
Registered in England No. 02527130

Investigation Layout Plan

Sheet: 1 of 1

Job No: 783212

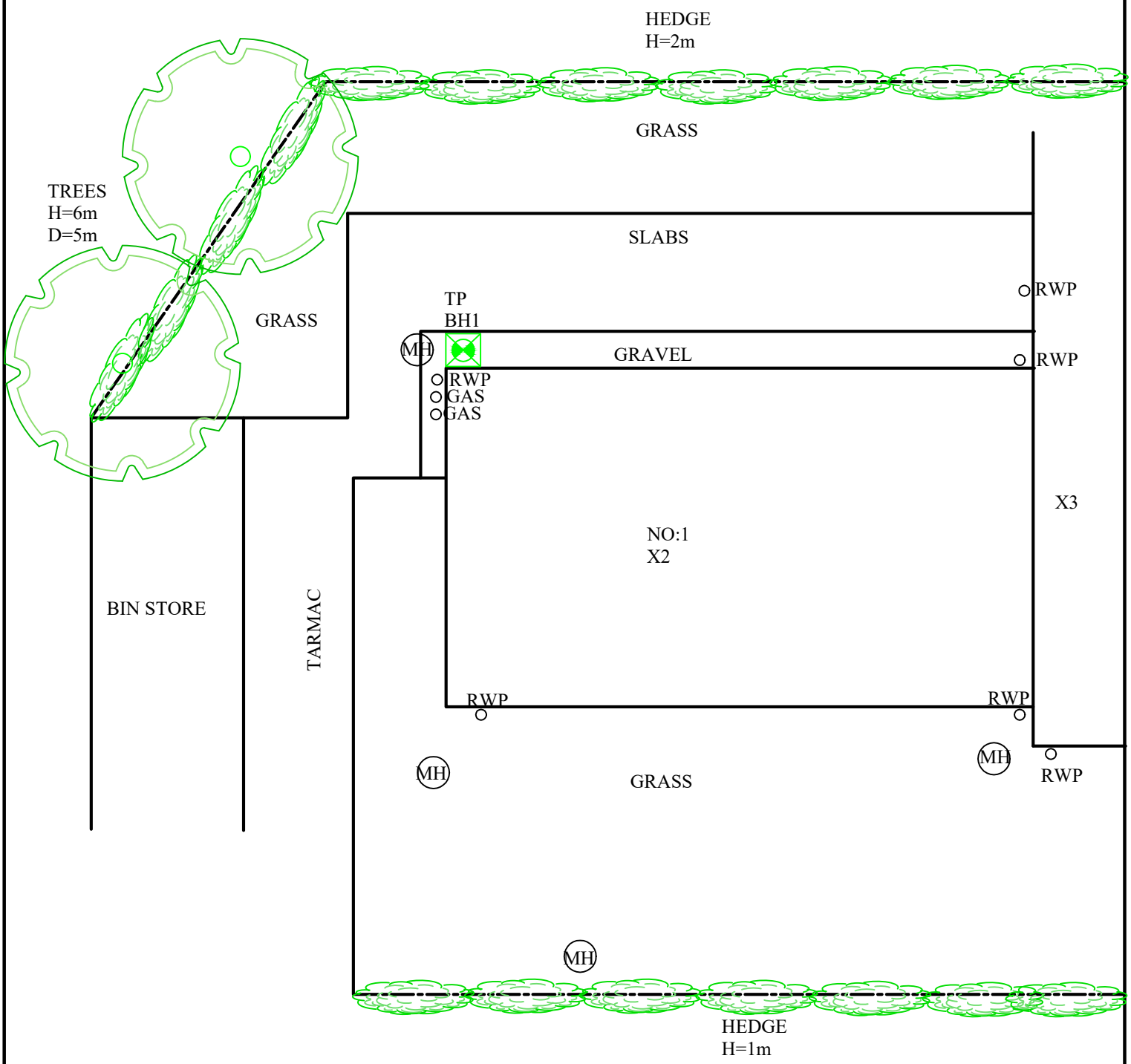
Date: 03/01/2024

Site: 1 Wheatstone Road

Work carried out for: Sedgwick International UK

AG	SL	EM
(SI)	(Checked)	(Drawn)

Weather: Dry



ON SITE TREE IDENTIFICATION FOR GUIDANCE ONLY. NOT AUTHENTICATED.

Remarks:

Scale: N.T.S.

Key:

Combined Gulley	RWWG
Manhole	MH
Rain Water Pipe	RWP
Rain Water Gulley	RWG
Soil Vent Pipe	SVP
Waste Gulley	WG
Waste Pipe	WP

Surface Water Drain

Foul Water Drain

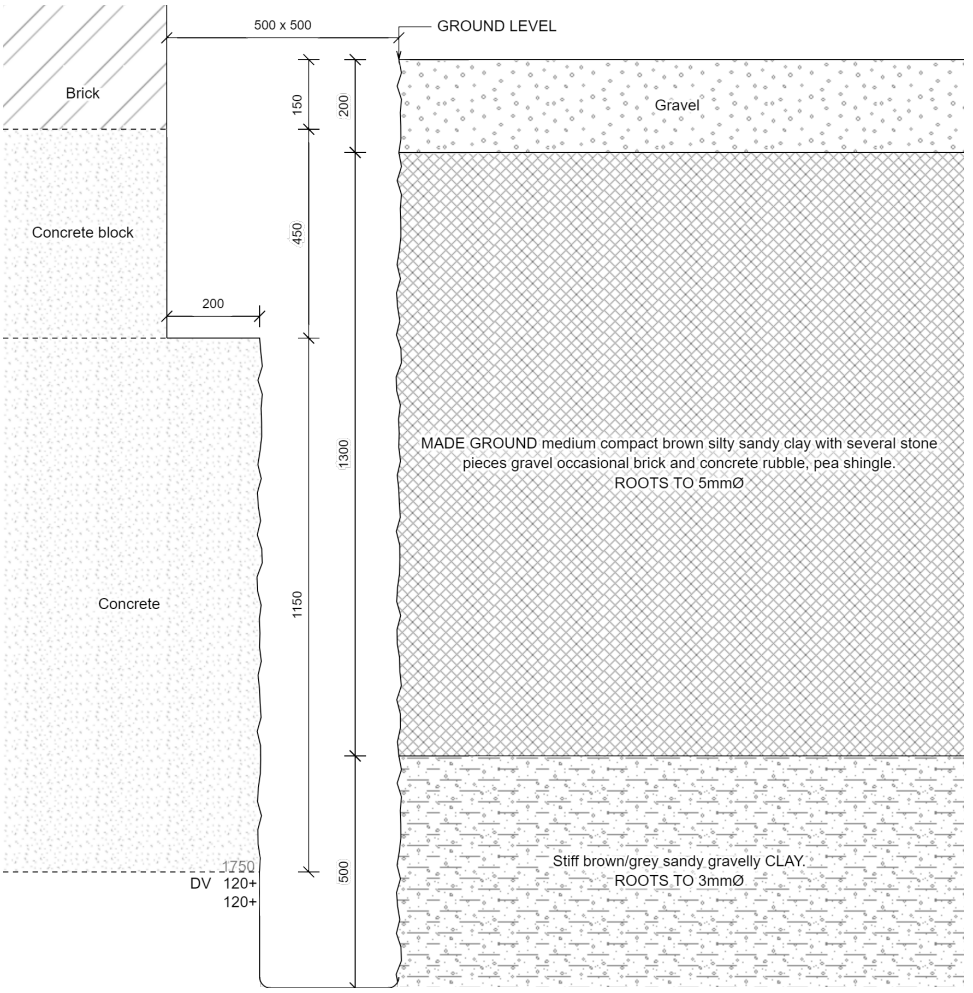
Tree / Bush (approx. ht in m)

Trial Pit

Borehole

O/D - Open Discharge

TEST REPORT:	**Trial Pit**		
REPORT NUMBER:	C1084382 / 282058.1.1.1		
TRIAL PIT REF:	TP1	DATE:	03/01/2024
CLIENT:	Sedgwick International UK	SITE:	1 WHEATSTONE ROAD
JOB NO:	783212	WEATHER:	Dry
EXCAVATION METHOD:	Hand tools		



For Strata below 2000mm see Bore Hole log

Curved steel pin driven 200mm under concrete foundation at 1750mm below ground level, TP hand excavated to 1200mm then extended with the aid of the hand auger to achieve USF depth.

Key:

D	Small disturbed sample	J	Jar sample
B	Bulk disturbed sample	V	Pilcon vane (kPa)
W	Water sample	M	Mackintosh probe
TDTD	Too dense to drive		

Remarks:
Test results reported relate only to the items tested.
This report shall not be reproduced except in full without approval of the Laboratory.
The laboratory does not apply a conformity statement to test reports as standard, unless specifically requested by the customer.

For and on behalf of CTS
Cara Kosma - Project Delivery Supervisor

Approved Signatory
Report date 04-Jan-24

Lawness Barns
Mountnessing Road
Billericay
Essex CM12 0TS

0343 227 8545
enquiries@constructiontesting.co.uk
www.constructiontesting.co.uk
END OF REPORT

Construction Testing Solutions Ltd.
Registered in England No. 05998333

Borehole	1		Sheet:	1 of 1	Site:	1 WHEATSTONE ROAD
			Job No:	783212		
			Date:	03/01/2024		
Boring Method:	Hand Auger		Ground Level:		Client:	SEDGWICK INTERNATIONAL UK
Diameter (mm):	75	Weather:	dry			

Depth	Soil Description	Thickness	Legend	Samples and Tests		
(m)				Depth	Type	Result
0.00	See Trial Pit	2.00				
2.00	Stiff brown-grey sandy slightly gravelly CLAY	1.70		2.00	DV	120+
						120+
				2.50	DV	120+
						120+
				3.00	DV	120+
						120+
				3.50	DV	120+
						120+
3.70	End of BH					

Remarks:
BH ends at 3.7m, too stiff/hard to auger, BH dry and open on completion. Roots observed to 3.7m.

Key:

		To Depth (m)	Max Dia (mm)
D - Disturbed Sample			
B - Bulk Sample			
W - Water Sample	Roots	3.70	2
J - Jar Sample	Roots		
V - Pilcon Shear Vane (kPa	Roots		
M - Mackintosh Probe	Depth to Water (m)		
TDTD - Too Dense To Drive			

Logged: AG	CK	Checked:	Approved:	Version V1.0 28/01/16	N.T.S.

SITE INVESTIGATION
LABORATORY TEST REPORT

SI REPORT NUMBER: 783212

CLIENT : CET Property Assurance (Sedgwick International UK)

SITE:
1 Wheatstone Road
Huntingdon
Cambridgeshire
PE29 1DE

DATE OF SITE VISIT:
03/01/2024

DATE RECEIVED BY LABORATORY:
03/01/2024

Approved by : .....M.Duffield.....
M Duffield - Operations Manager

DATE REPORTED: 19-Jan-2024

The testing on this report has been subcontracted, see Summary for testing Laboratory details

Our Ref : 783212

Laboratory Summary Results

Date Sampled: 03/01/2024

Location : 1 Wheatstone Road

Date Received : 03/01/2024

Client: CET Property Assurance (Sedgwick International UK)

Date Tested : 12/01/2024

Address: CET, Unit 4, Boundary Court, Willow Farm Business Park, Castle Donington, DE74 2NN

Date of Report : 19/01/2024

Sample Ref		Type	# Moisture Content	# Soil Fraction > 0.425mm	# Liquid Limit	# Plastic Limit	~ Plasticity Index	~ Liquidity * Index	~ Modified * Plasticity Index	~ Soil * Class	# Filter Paper Contact Time	# Soil Sample Suction	# Oedometer Strain	~ Estimated * Heave Potential (Dd)	In situ * Shear Vane Strength	Organic * Content	pH Value	Sulphate Content		* Class
TP/BH No	Depth (m)		(%) *[1]*	(%) *[2]*	(%) *[3]*	(%) *[4]*	(%) *[5]*	*[5]*	(%) [6]	*[7]*	(d)	(kPa) [8]	*[9]*	*(mm)[10]*	(kPa) *[11]*	(%)*[12]*	*[13]*	SO_3 (g/l) * *[14]*	SO_4 (mg/l) *[15]*	*[16]*
1	U/S 1.75	D	21	22	43	22	21	-0.05	16	CI	7	57.2			> 120					
	2.0	D	24	14	53	24	29	0.00	25	CH	7	420			> 120					
	2.5	D	16	20	49	23	26	-0.26	21	CI	TOO HARD				> 120					
	3.0	D	26	16											> 120					
	3.5	D	24	20	51	22	29	0.08	23	CH	7	929			> 120					

Test Methods / Notes

[1] BS 1377 : Part 2 : 1990, Test No 3.2

[2] Estimated if <5%, otherwise measured

[3] BS 1377 : Part 2 : 1990, Test No 4.4

[4] BS 1377 : Part 2 : 1990, Test No 5.3

[5] BS 1377 : Part 2 : 1990, Test No 5.4

[6] BRE Digest 240 : 1993

[7] BS 5930 : 2018 : Figure 8 - Plasticity Chart for the classification of fines soils

[8] Building Research Establishment Information Paper 4/93

[9] In Accordance with BS 1377-5 : 1990 : Clause 3

[10] Estimated Heave Potential (Dd)

[11] Values of shear strength were determined in situ by CTS using a Pilcon hand vane or Geonor vane (GV).

[12] BS 1377 : Part 3 : 2018 + A1 2021 Clause 4 - *Tested By CTS Leicester*

[13] BS 1377 : Part 3 : 2018 + A1 2021 Clause 12 - *Tested By CTS Leicester*

[14] Sulphate content as SO3 as required by BS 1377: Part 3: 1990 has been provided for information purposes - *Tested By CTS Leicester*

[15] BS 1377 : Part 3 : 2018 + A1 2021 Clause 7.6 - Tested By CTS Leicester

[16] BRE Special Digest One (Concrete in Aggressive Ground) August 2005 Note that if the SO4 content falls into the DS-4 or DS-5 class, it would be prudent to consider the sample as falling into the DS-4M or DS-5M class respectively unless water soluble magnesium testing is undertaken to prove otherwise.

PSD Chart - BS 1377: Part 2 : 1990, Test No 9.2

Key

D	Disturbed sample (small)
B	Disturbed sample (bulk)
U	Undisturbed sample
W	Groundwater sample
ENP	Essentially Non-Plastic by inspection
U/S	Underside of Foundation

~ Calculations performed using subcontracted data.

* These tests are not UKAS accredited

 # These tests have been subcontracted and carried out by PSL (Part of the Phenna Group)

Full reports can be provided upon request.

Test results reported relate only to the items tested.

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The laboratory does not apply a conformity statement to test reports as standard, unless specifically requested by the customer.

Opinions and interpretations expressed herein are outside of the scope of UKAS accreditation.

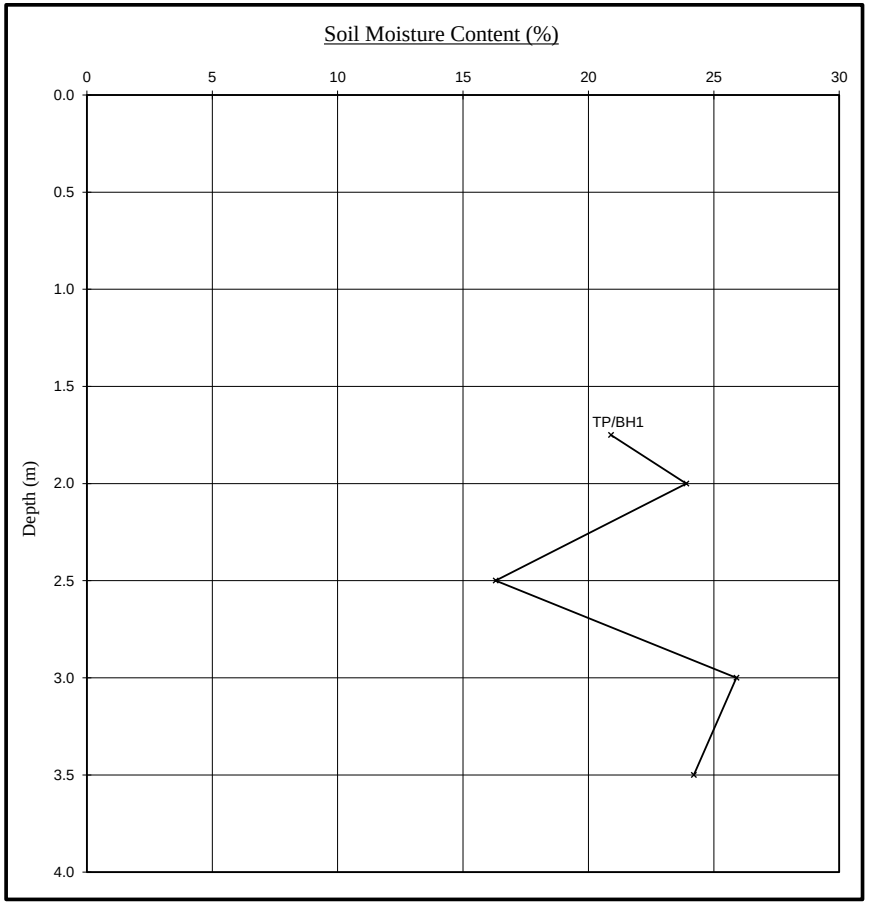
Version: BH V1 SUBCON - 28.03.2023

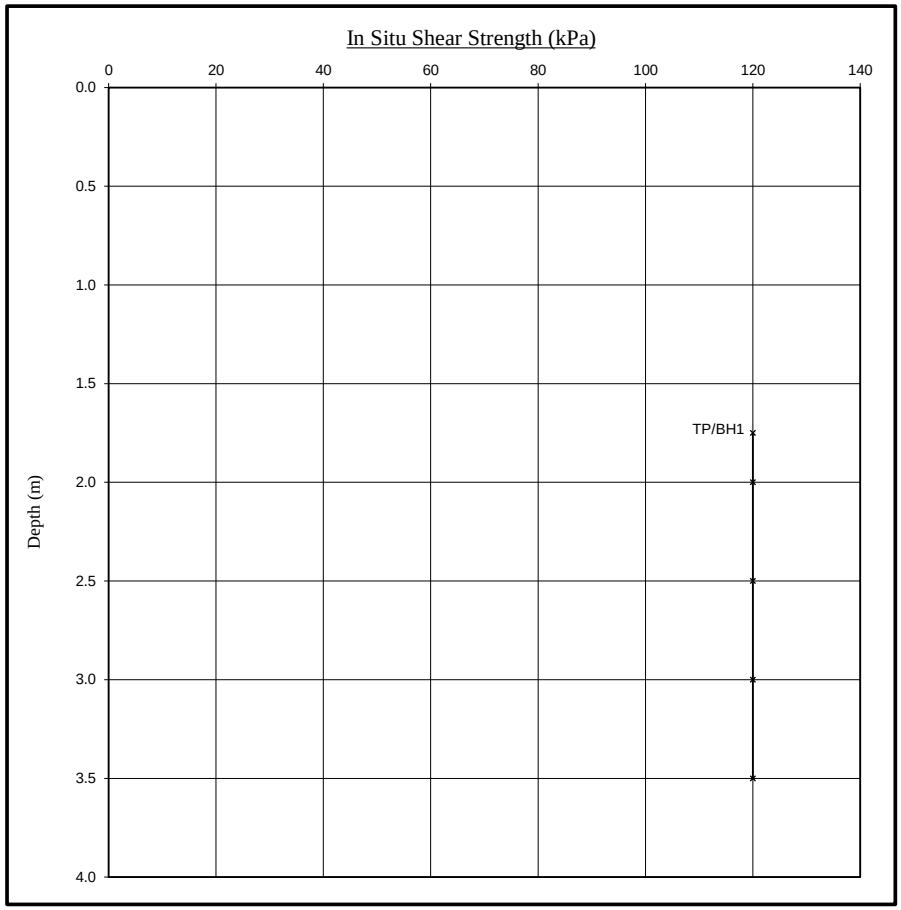
Moisture Content Profiles

Shear Strength Profiles

Our Ref : 783212
Location : 1 Wheatstone Road
Work carried out for: CET Property Assurance (Sedgwick International UK)

Date Sampled : 03/01/2024
Date Received : 03/01/2024
Date Tested : 12/01/2024
Date of Report : 19/01/2024





Notes
1. If plotted, 0.4 LL and PL+2 (after Driscoll, 1983) should only be applied to London Clay (and similarly overconsolidated clay) at shallow depths.
2. Unless specifically noted the profiles have not been related to a site datum.

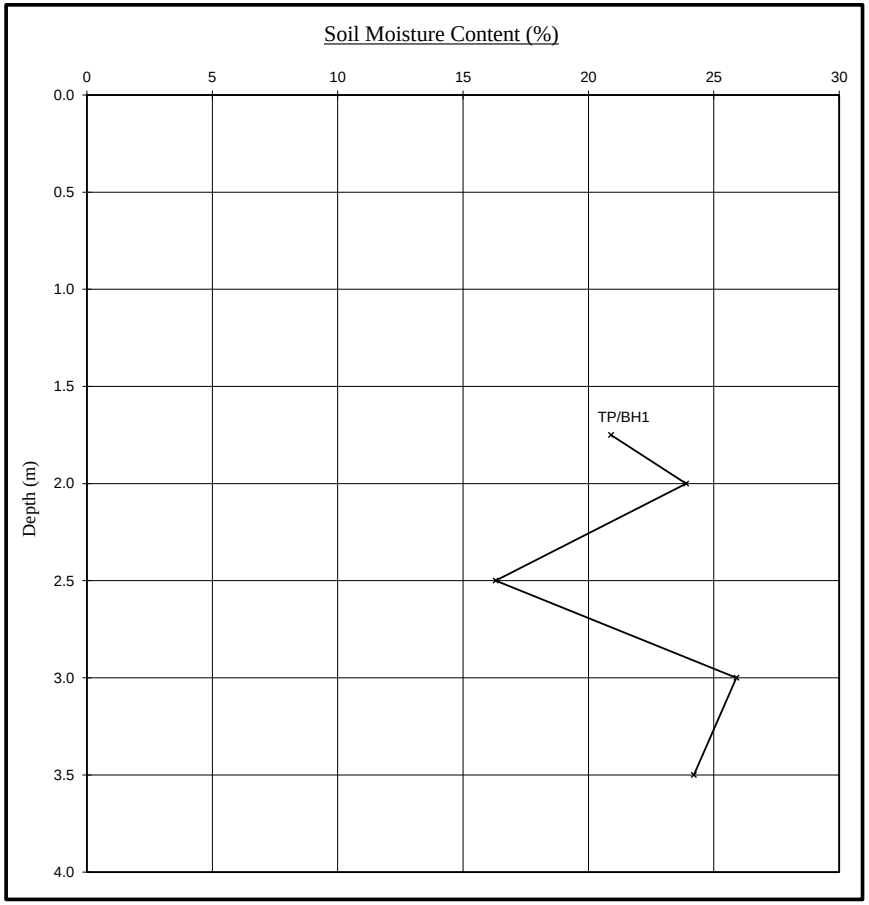
Note
1. Unless otherwise stated, values of Shear Strength were determined in situ by CTS using a Pilcon Hand Vane the calibration of which is limited to a maximum reading of 130 kPa.
2. Unless specifically noted the profiles have not been related to a site datum.

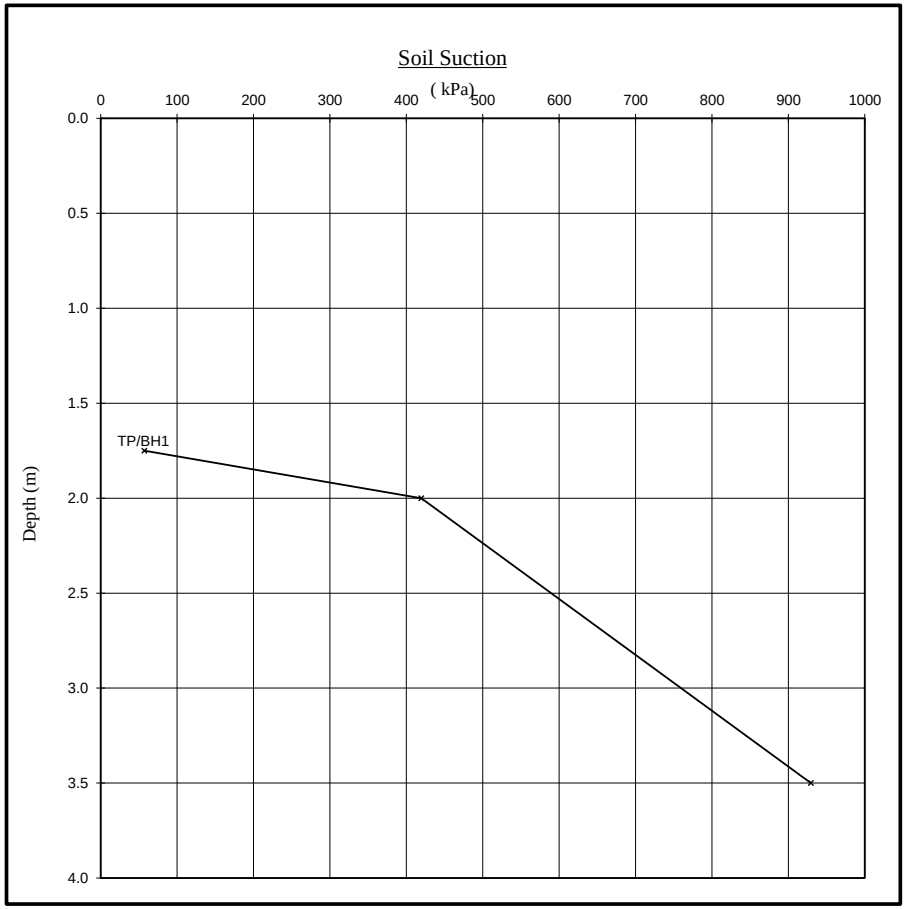
Moisture Content Profiles

Soil Suction Profiles

Our Ref : 783212
Location : 1 Wheatstone Road
Work carried out for: CET Property Assurance (Sedgwick International UK)

Date Sampled : 03/01/2024
Date Received : 03/01/2024
Date Tested : 12/01/2024
Date of Report : 19/01/2024





Notes
1. If plotted, 0.4 LL and PL+2 (after Driscoll, 1983) should only be applied to London Clay (and similarly overconsolidated clay) at shallow depths.
2. Unless specifically noted the profiles have not been related to a site datum.

Note
When shown, the theoretical equilibrium suction profiles are based on conventional assumptions associated with London Clay (and similarly overconsolidated clays) at shallow depths. Note that the sample disturbance component is dependant on the method of sampling and any subsequent recompaction. The above plots show this to be 100kPa which is the value suggested by the BRE on the basis of their limited number of tests on recompacted samples. This may or may not be appropriate in this instance and judgement should be exercised.

EPSL
Independent Scientific Analysis

European Plant Science Laboratory

Construction Testing Solutions
4 Oak Spinney Park
Ratby Lane
Leicester Forest East
Leicestershire
LE3 3AW

Intec
Parc Menai, Bangor,
Gwynedd, North Wales
LL57 4FG
Tel: 01248 672652
Fax: 01248 672601

ROOT IDENTIFICATION

1 Wheatstone Road

Client Reference: 783212
Report Date: 10 January 2024
Our Ref: R56562

Sub Sample	Species Identified		Root Diameter	Starch
TP1:				
USF	*Fraxinus* spp.	1	2 mm	Abundant
BH1:				
to 3.7m	*Fraxinus* spp.	2	2 mm	Abundant

Comments:

1 - Plus 3 others also identified as *Fraxinus* spp.
2 - Plus 2 others also identified as *Fraxinus* spp.

Fraxinus spp. include common ash.

Signed: R. Shaw

Unless we are otherwise instructed in writing, the above sample material will normally be disposed of 6 years after the date of this report.